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Turbulent two-dimensional shallow water equations and their numerical solution

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Abstract Free surface flow of water over a shallow rough bed is characteristically turbulent due to disturbances generated by the bed resistance and diverse causes. The paper presents a derivation of the basic equations in two dimensions and their numerical solution by an extension of the method developed earlier for flow in one dimension. Starting from the three-dimensional Reynolds Averaged Navier-Stokes equations, the equations of continuity and horizontal momenta are depth averaged to derive three equations for the free surface elevation ζ and the horizontal, depth averaged velocity components (U, V). Certain closure assumptions are required for derivation of the equations. Principally, the viscous stresses are neglected, while the Reynolds stresses are assumed to depend on the vertical coordinate z only for the shearing flow over the x, y-plane representing the plane bed. Secondly, it is assumed that the instantaneous horizontal components of velocity (u, v) follow the 1/pth (p = 7) power law of variation in the z-direction, in liu of the logarithmic law of the wall. For numerical solution of the three nonlinear equations of continuity and momenta, the equations are reformulated in terms of the primitive "discharge" components (Q, R) of the velocity (U, V), showing that Q and R can be functions of ζ alone. The transformed equation of continuity is treated by the Lax-Richtmyer method. The two momentum equations on the other hand, transform in to two coupled second degree equations in the derivatives of Q and R, which decouple in the important case of quasilinear straight crested waves on the water surface. The decoupled equations are numerically solved by the iterative modified Euler method, and illustrated by application to an initial elevation of a model for tsunami propagation. Both of the Lax-Richtmyer and the iterative modified Euler method are of second order. The numerical method is general and applicable in other cases such as gradual atmospheric flows over the globe.

 $\label{eq:keywords} \textbf{Keywords} \quad \text{Two-dimensional shallow water equations} \cdot \text{Turbulence} \cdot \text{RANS equations} \cdot \text{Depth averaging} \cdot \text{Discharge} \cdot \text{Lax-Richtmyer method} \cdot \text{Modified Euler Method}$

1 Introduction

Shallow water equations in two dimensions are useful in oceanic and atmospheric flows as the aspect ratio of the horizontal spread to vertical height is very large. In the development of the equations, for oceanic flows such as tsunamis, tides, storm surges and oceanic circulations, the fluid pressure at a point is considered hydrostatic, even though the free surface may have vertical motion. The formulation of the governing equations of mass conservation and the horizontal components of momentum turn out to be in terms of the surface elevation ζ and the depth averaged components of the horizontal velocity (U, V). The equations neglecting the wind forces on the free surface of the water are the equation of continuity

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$$\frac{\partial \zeta}{\partial t} + \frac{\partial (\zeta U)}{\partial x} + \frac{\partial (\zeta V)}{\partial y} = 0 \tag{1}$$

and the momentum equations in the x, y-directions as

$$\frac{\partial U}{\partial t} + U \frac{\partial U}{\partial x} + V \frac{\partial U}{\partial y} + g \frac{\partial \zeta}{\partial x} + \tau_{0x} = 0$$
 (2)

$$\frac{\partial V}{\partial t} + U \frac{\partial V}{\partial x} + V \frac{\partial V}{\partial y} + g \frac{\partial \zeta}{\partial y} + \tau_{0y} = 0$$
 (3)

where (τ_{0x}, τ_{0y}) are the components of bed resistance that are usually modeled in the hydraulics literature by either the Chezy or by the Manning empirical formulae of the following forms:

$$\tau_{0x} = \frac{gU\sqrt{U^2 + V^2}}{C^2 \zeta}, \ \tau_{0y} = \frac{gV\sqrt{U^2 + V^2}}{C^2 \zeta}, \ \text{(Chezy)}$$
(4a)

$$\tau_{0x} = \frac{m^2 g U \sqrt{U^2 + V^2}}{\zeta^{1/3}}, \ \tau_{0y} = \frac{m^2 g V \sqrt{U^2 + V^2}}{\zeta^{1/3}}, \ (Manning)$$
 (4b)

where C and m are, respectively, the Chezy and the Manning coefficients that depend on the roughness of the bed, and g the acceleration due to gravity (Mader [1], p. 27, in different notations under the Chezy assumption for τ_{0x} and τ_{0y}). The nonlinear partial differential equations (1), (2) and (3) are also treated by Mader [1] using first-order finite differences and applying them to a number of applications, in particular to tsunamis.

Among the more advanced methods, Fennema and Chowdhry [2,3] have, respectively, given implicit and explicit type of finite difference methods. The former is of Beam-Warming type and the latter of MacCormack-Gobuti type, both being second-order accurate in space as well as time. On the other hand, Wilders et. al. [4] and Casulli [5] develop fully implicit splitting and semi-implicit finite difference methods for the equations. In recent years, Ouyang et al. [6] present a MacCormack-TVD finite difference scheme for solution of the equations, whereas Fu and Hodges [7] present a time-centered split-implicit method, citing some other works. Much earlier, however, Katopodes and Strelkoff [8] treat the equations differently by the method of characteristic surfaces. Finite volume methods have also been proposed. Alcrudo and Garcia-Navarro [9] give a high resolution Godunov-type scheme, while Peng and Tang [10] present a method for which a Godunov-type scheme is used for solving the Riemann problem. Garcia-Navarro et al. [11] have also presented a multidimensional upwind scheme. Oceanic circulation models based on the two-dimensional shallow water equations are also commercially available as software packages.

Here, in this paper, the shallow water equations in two dimensions are developed taking in to consideration the turbulence generated by bed friction and other causes. The methodology is a generalization of the one given by Bose and Dey [12] for the case of one-dimensional open channel flows. Beginning with the threedimensional RANS equations, it is assumed that the Reynolds stress components at a point depend on the vertical z coordinate of the point only in a flow, which is essentially of shearing type parallel to the bed. It is then argued following Schlichting [13] that the horizontal components of the time averaged instantaneous velocity (u, v) vary as $z^{1/p}$ where p = 7. The proportionality constants appearing in the expressions for u and v are then written in terms of the respective depth-averaged horizontal velocities (U, V), which are functions of the horizontal coordinates (x, y) only. The one-seventh power law is used here instead of the better known logarithmic law of the wall, as the former leads to explicit depth averaged equations for the flow. An expression for the time averaged vertical velocity w in terms of U, V and z follows from the equation of continuity. Similarly, depth averaging of the vertical component of the momentum equation subject to the assumption of linear variation of the stream surface with respect to z yields the expression for the time averaged pressure \bar{p} , which contains apart from the hydrostatic pressure, nonlinear terms on account of vertical instantaneous and convective accelerations. Depth averaging of the continuity and the two horizontal momentum equations then yield the depth averaged continuity and momentum equations in terms of ζ , U and V. Numerical treatment of the nonlinear equations is then developed generalizing that given by Bose [14] for one-dimensional flows. Expressing the three equations in terms of "discharge" components $Q = \zeta U$ and $R = \zeta V$ parallel to the x and y axes, it is shown that the two quantities can be functions of ζ alone for gradually varying flows of the long wave type, that is to say, $Q = \overline{F}(\zeta)$ and $R = G(\zeta)$. The two nonlinear momentum equations then reduce to two coupled second degree equations in $F'(\zeta)$ and $G'(\zeta)$, where the prime denotes differentiation with respect to ζ . In the important case of quasilinear nearly straight crested waves parallel to the y-axis, the equations decouple in to a quadratic equation in $F'(\zeta)$ and an explicit equation for $G'(\zeta)$, where the primes in the two functions F and G denote differentiation with respect to ζ . The averaged continuity equation on the other hand reduces to a standard form of conservation equation, which is treated by the second-order Lax–Richtmyer method [16], where in the first-order pair of ordinary differential equations for $F(\zeta)$ and $G(\zeta)$ is solved by the second-order iterative modified Euler method (Bose [15]. The method is illustrated by application to an initial elevation form proposed in Howe [17], for a model tsunami propagation problem.

2 The Turbulent 2D Shallow Water Equations

The two-dimensional turbulent shallow water free surface flow over a plane horizontal bed is governed by the general RANS equations. As the free surface in general can be undulating during the flow, the plane bed is conveniently chosen as the x, y-plane with the z-axis drawn vertically upwards, with a mean water depth of h units. Due to turbulence, the velocity at a point (x, y, z) in the fluid consists of the time averaged components (u, v, w) and instantaneous fluctuations (u', v', w'). The time averaged components satisfy the RANS equation of continuity

$$\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} + \frac{\partial w}{\partial z} = 0 \tag{5}$$

for incompressible fluids. The fluctuating velocity components satisfy a similar equation. The RANS momentum equations (Schlichting [13], p. 562) contain contributions of (u, v, w), that of the pressure \bar{p} , and the viscous and Reynolds stresses. Moreover, as the motion over the horizontal bed is essentially of shearing type, the viscous and Reynolds stress components become practically independent of x and y. Hence, neglecting the contribution of the normal viscous stress in the vertical direction, the RANS momentum equations read as

$$\frac{\partial u}{\partial t} + u \frac{\partial u}{\partial x} + v \frac{\partial u}{\partial y} + w \frac{\partial u}{\partial z} = -\frac{1}{\rho} \frac{\partial \bar{p}}{\partial x} + \frac{1}{\rho} \frac{\partial \tau_{zx}}{\partial z} + \frac{\mu}{\rho} \frac{\partial^2 u}{\partial z^2}$$
 (6)

$$\frac{\partial v}{\partial t} + u \frac{\partial v}{\partial x} + v \frac{\partial v}{\partial y} + w \frac{\partial v}{\partial z} = -\frac{1}{\rho} \frac{\partial \bar{p}}{\partial y} + \frac{1}{\rho} \frac{\partial \tau_{zy}}{\partial z} + \frac{\mu}{\rho} \frac{\partial^2 v}{\partial z^2}$$
 (7)

$$\frac{\partial w}{\partial t} + u \frac{\partial w}{\partial x} + v \frac{\partial w}{\partial y} + w \frac{\partial w}{\partial z} = -g - \frac{1}{\rho} \frac{\partial \bar{p}}{\partial z} - \frac{\partial \overline{w'^2}}{\partial z}$$
(8)

where ρ = density, μ = dynamical coefficient of viscosity, g = acceleration due to gravity, and τ_{zx} = $-\rho \overline{w'u'}$, $\tau_{zy} = -\rho \overline{w'v'}$ are Reynolds stresses in which the overbars denote time averages. Equations (5),(6), (7) and (8) describing the flow are under determined, and so additional conditions have to be incorporated.

3 The 1/pth Power Law

For open channel flow in one direction only, say the x-direction, it is known that u varies slowly as $z^{1/p}$, where p = 7 (Schlichting [13], p. 590; Bose and Dey [12]). Similarly, in the present generalization, one can assume that

$$u = \frac{p+1}{p}U(x, y, t) \left(\frac{z}{\zeta}\right)^{1/p}, \quad v = \frac{p+1}{p}V(x, y, t) \left(\frac{z}{\zeta}\right)^{1/p}$$
(9)

where the velocity (U, V) equals the depth average of (u, v), and $\zeta(x, y, t)$ is the surface elevation over the point (x, y) on the bed. Accordingly, from Eq. (5) by integration

$$w = -\int_0^z \left(\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y}\right) dz = -\left(\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y}\right) \frac{z^{1/p+1}}{\zeta^{1/p}}$$
(10)

where it is assumed for simplicity that the forward flux $(\partial U/\partial x, \partial V/\partial y)$ of the flow is much greater than the slope factor (U/ζ) $(\partial \zeta/\partial x, \partial \zeta/\partial y)$ of the free surface, so that the latter terms become insignificant compared to the term written in equation (10). Physically, the approximation (10) implies that as the rate of expansion $\partial U/\partial x + \partial V/\partial y$ increases as the vertical velocity w decreases, which apparently is quite plausible due to the

mass conservation principle. The solution forms (9) and (10) are next used in Eqs. (5)–(8). The convective acceleration term in Eq. (8), using Eq. (5) becomes

$$u\frac{\partial w}{\partial x} + v\frac{\partial w}{\partial y} + w\frac{\partial w}{\partial z} = u\frac{\partial w}{\partial x} + v\frac{\partial w}{\partial y} - w\left(\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y}\right) = u^2\frac{\partial}{\partial x}\left(\frac{w}{u}\right) + v^2\frac{\partial}{\partial y}\left(\frac{w}{v}\right)$$

$$= u^2\frac{\partial^2 z_1}{\partial x^2} + v^2\frac{\partial^2 z_1}{\partial y^2}$$

$$(11)$$

wherein $z=z_1(x,y)$ represents the stream surface passing through the point (x,y,z). The two partial derivatives $\frac{\partial^2 z_1}{\partial x^2}$ and $\frac{\partial^2 z_1}{\partial y^2}$ in Eq. (11) are proportional to the curvature of the surface $z=z_1(x,y,t)$ at time t and are assumed to linearly vary with z, as in the Boussinesq approximation of open channel flows. Then,

$$\frac{\partial^2 z_1}{\partial x^2} = \frac{z}{\zeta} \frac{\partial^2 \zeta}{\partial x^2}, \quad \frac{\partial^2 z_1}{\partial y^2} = \frac{z}{\zeta} \frac{\partial^2 \zeta}{\partial y^2}$$
 (12)

Hence using Eq. (9), the convective acceleration (11) becomes

$$u\frac{\partial w}{\partial x} + v\frac{\partial w}{\partial y} + w\frac{\partial w}{\partial z} = \left(\frac{p+1}{p}\right)^2 \left(\frac{z}{\zeta}\right)^{2/p+1} \left(U^2 \frac{\partial^2 \zeta}{\partial x^2} + V^2 \frac{\partial^2 \zeta}{\partial y^2}\right)$$
(13)

Inserting the expression (13) in Eq. (8) and integrating over depth, one gets

$$\frac{p}{\rho} = -\int_{\zeta}^{z} \frac{\partial w}{\partial t} dz - \frac{p+1}{2p} \zeta \left\{ \left(\frac{z}{\zeta} \right)^{2/p+2} - 1 \right\} \left(U^{2} \frac{\partial^{2} \zeta}{\partial x^{2}} + V^{2} \frac{\partial^{2} \zeta}{\partial y^{2}} \right) - \overline{w'^{2}} - g \left(z - \zeta \right) + \frac{\bar{p}_{0}}{\rho}$$
(14)

where \bar{p}_0 is the constant pressure over the free surface $z = \zeta$. Thus the formal expression for the pressure is also obtained. In Eq. (14), the last two terms represent the contribution of hydrostatic pressure and the rest represents the additional contribution due to turbulence.

4 Depth Averaging

As the water depth is shallow, it is appropriate to consider the depth averages of the continuity and horizontal momentum equations (5), (6) and (7). Integration of Eq. (5) with respect to z over (0, ζ) yields the depth averaged continuity equation (1). Treating the momentum equations (6) and (7) in a similar manner, the former leads to the equation

$$\frac{\partial}{\partial t}(\zeta U) + \frac{\partial}{\partial x}\left(\int_0^\zeta u^2 dz\right) + \frac{\partial}{\partial y}\left(\int_0^\zeta uv \,dz\right) + \frac{1}{\rho}\int_0^\zeta \frac{\partial \bar{p}}{\partial x} \,dz + \frac{\tau_{0x}}{\rho} = 0$$
 (15)

where τ_{0x} = bed resistance parallel to the *x*-axis. The representations (9) yield for the first two integrals of Eq. (15) as

$$\int_0^{\zeta} u^2 dz = \alpha \zeta U^2, \quad \int_0^{\zeta} uv \, dz = \alpha \zeta UV$$
 (16)

where $\alpha = (p+1)^2/[p(p+2)]$. On the other hand,

$$\frac{1}{\rho} \int_0^{\zeta} \frac{\partial \bar{p}}{\partial x} dz = \frac{\partial}{\partial x} \int_0^{\zeta} \frac{\bar{p}}{\rho} dz$$
 (17)

as $\bar{p}(x, y, \zeta) = 0$ on the free surface. Hence using Eq. (10) with Eqs. (5),

$$\frac{1}{\rho} \int_{0}^{\zeta} \frac{\partial \bar{p}}{\partial x} dz = \gamma \frac{\partial}{\partial x} \left[\zeta^{2} \left(U^{2} \frac{\partial^{2} \zeta}{\partial x^{2}} + V^{2} \frac{\partial^{2} \zeta}{\partial y^{2}} \right) \right] - \delta \frac{\partial}{\partial x} \left[\zeta^{3} \frac{\partial}{\partial t} \left(\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} \right) - \frac{\zeta^{3}}{p} \frac{\partial \zeta}{\partial t} \left(\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} \right) \right] + g \frac{\partial \zeta}{\partial x} \tag{18}$$

in which, $\gamma = (p+1)^2/[p(3p+2)]$, $\delta = p/(3p+1)$, and the Reynolds normal stress term $\partial(\overline{w'^2})/\partial x$ is negligible as assumed earlier. The contribution of the bed shear term $\tau_{0x} = -\mu (\partial u/\partial z)_{z=0}$ in Eq. (15) can only be estimated. Here the Manning's formula (4b) is adopted as the coefficient m appearing in the formula has lesser variation with respect to variation with the roughness of the bed surface. Equation (15) therefore becomes

$$\frac{\partial}{\partial t}(\zeta U) + \alpha \frac{\partial}{\partial x}(\zeta U^{2}) + \alpha \frac{\partial}{\partial y}(\zeta U V) + \gamma \frac{\partial}{\partial x} \left[\zeta^{2} \left(U^{2} \frac{\partial^{2} \zeta}{\partial x^{2}} + V^{2} \frac{\partial^{2} \zeta}{\partial y^{2}} \right) \right]
- \delta \frac{\partial}{\partial x} \left[\zeta^{3} \frac{\partial}{\partial t} \left(\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} \right) - \frac{\zeta^{2}}{p} \frac{\partial \zeta}{\partial t} \left(\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} \right) \right] + g \zeta \frac{\partial \zeta}{\partial x} + \frac{m^{2} g U \sqrt{U^{2} + V^{2}}}{\zeta^{1/3}} = 0$$
(19)

In a similar manner Eq. (7) leads to the averaged equation

$$\frac{\partial}{\partial t}(\zeta V) + \alpha \frac{\partial}{\partial y}(\zeta V^{2}) + \alpha \frac{\partial}{\partial x}(\zeta U V) + \gamma \frac{\partial}{\partial y} \left[\zeta^{2} \left(U^{2} \frac{\partial^{2} \zeta}{\partial x^{2}} + V^{2} \frac{\partial^{2} \zeta}{\partial y^{2}} \right) \right]
- \delta \frac{\partial}{\partial y} \left[\zeta^{3} \frac{\partial}{\partial t} \left(\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} \right) - \frac{\zeta^{2}}{p} \frac{\partial \zeta}{\partial t} \left(\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} \right) \right] + g \zeta \frac{\partial \zeta}{\partial y} + \frac{m^{2} g V \sqrt{U^{2} + V^{2}}}{\zeta^{1/3}} = 0$$
(20)

Equations (1), (19) and (20) are then the depth averaged continuity and the x, y components of the momentum equation.

5 Simplification for Numerical Solution

Since p=7 approximately, $\alpha=64/63\approx 1.01587\cdots\approx 1$, $\gamma=64/161=0.39751\cdots\approx 0.4=2/5$, and $\delta=7/22$. The approximation for α leads to familiar convective acceleration terms of Eqs. (2) and (3), as can be seen in the Eqs. (23) and (24) derived in the following. The approximation for γ is introduced to bring some elegance to the basic equations. The highly nonlinear fifth term in both of the Eqs. (19) and (20) arises due to the instantaneous vertical acceleration $\partial w/\partial t$. For developing the numerical solution method, it is first noted that the two terms are horizontal gradients that need not be large. Hence, one obtains by iteration of (19) and (20), the first-order approximations

$$\frac{\partial U}{\partial t} \approx -g \frac{\partial \zeta}{\partial x}, \quad \frac{\partial V}{\partial t} \approx -g \frac{\partial \zeta}{\partial y} \quad \text{and} \quad \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} \approx -\frac{1}{\zeta} \frac{\partial \zeta}{\partial t}$$
 (21)

The left-hand side quantities of (21) are then inserted in the δ -terms of Eqs. (19) and (20). Thus Eq. (19) becomes

$$\zeta \frac{\partial U}{\partial t} - \zeta U \left(\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} \right) + \frac{\partial}{\partial x} \left[\zeta \left\{ U^2 - \frac{1}{22} \left(\frac{\partial \zeta}{\partial t} \right)^2 \right\} \right] + \frac{\partial}{\partial y} (\zeta U V) + \frac{2}{5} \frac{\partial}{\partial x} \left[\zeta^2 \left(U^2 \frac{\partial^2 \zeta}{\partial x^2} + V^2 \frac{\partial^2 \zeta}{\partial y^2} \right) \right] \\
+ \frac{7g}{22} \frac{\partial}{\partial x} \left[\zeta^3 \left(\frac{\partial^2 \zeta}{\partial x^2} + \frac{\partial^2 \zeta}{\partial y^2} \right) \right] + g\zeta \frac{\partial \zeta}{\partial x} + \frac{m^2 g U \sqrt{U^2 + V^2}}{\zeta^{1/3}} = 0$$
(22)

A further approximation is possible in Eq. (22) if one assumes that $U^2 >> \frac{1}{22} \left(\frac{\partial \zeta}{\partial t}\right)^2$, as the horizontal velocity is much larger than the instantaneous vertical velocity of the free surface. With this assumption, Eq. (22) simplifies to

$$\frac{\partial U}{\partial t} + U \frac{\partial U}{\partial x} + V \frac{\partial U}{\partial y} + \frac{2}{5\zeta} \frac{\partial}{\partial x} \left[\zeta^2 \left(U^2 \frac{\partial^2 \zeta}{\partial x^2} + V^2 \frac{\partial^2 \zeta}{\partial y^2} \right) \right] + \frac{7g}{22\zeta} \frac{\partial}{\partial x} \left[\zeta^3 \left(\frac{\partial^2 \zeta}{\partial x^2} + \frac{\partial^2 \zeta}{\partial y^2} \right) \right]
+ g \frac{\partial \zeta}{\partial x} + \frac{m^2 g U \sqrt{U^2 + V^2}}{\zeta^{1/3}} = 0$$
(23)

Similarly Eq. (20) reduces to

$$\frac{\partial V}{\partial t} + U \frac{\partial V}{\partial x} + V \frac{\partial V}{\partial y} + \frac{2}{5\zeta} \frac{\partial}{\partial y} \left[\zeta^2 \left(U^2 \frac{\partial^2 \zeta}{\partial x^2} + V^2 \frac{\partial^2 \zeta}{\partial y^2} \right) \right] + \frac{7g}{22\zeta} \frac{\partial}{\partial y} \left[\zeta^3 \left(\frac{\partial^2 \zeta}{\partial x^2} + \frac{\partial^2 \zeta}{\partial y^2} \right) \right]
+ g \frac{\partial \zeta}{\partial y} + \frac{m^2 g V \sqrt{U^2 + V^2}}{\zeta^{1/3}} = 0$$
(24)

The continuity and momentum equations (1) and (23) and (24) are in physical dimensions. In order to nondimensionalize the equations, let the variables x, y, t, ζ and U, V be temporarily written as \hat{x} , \hat{y} , \hat{t} , $\hat{\zeta}$ and \hat{U} , \hat{V} , respectively; then writing

$$\hat{x} = hx, \hat{y} = hy, \hat{\zeta} = h\zeta, \hat{t} = t\sqrt{h/g}, \text{ and } \hat{U} = U\sqrt{gh}, \hat{V} = V\sqrt{gh}$$
 (25)

in which h is the mean depth of water; then the variables x, y, ζ , t and U, V become nondimensional. By the transformations (25), Eq. (1) remains unchanged in form, while the factor g drops out from the Eqs. (23) and (24). Hereafter the nondimensional form of these equations is only considered. Equation (23) with V = 0 and $\partial(\cdot)/\partial y = 0$ corresponds to the one-dimensional case studied by Bose [14], but here it is slightly simpler in form.

6 Transformation and Localization Assumption

In order to develop the numerical procedure for solving the Eqs. (1) and (23) and (24), it is noted that the continuity equation (1) relates the instantaneous vertical velocity of the free surface at (x, y) to the gradient of the discharge (Q, R) in the x, y-directions defined by

$$Q := \zeta U, \quad R := \zeta V \tag{26}$$

so that Eq. (1) in compact substricted notation for partial derivatives becomes

$$\zeta_t + Q_x + R_y = 0 \tag{27}$$

In terms of Q, R one has

$$U = \frac{Q}{\zeta}, \quad U_t = \frac{Q_t}{\zeta} + \frac{Q}{\zeta^2} (Q_x + R_y), U_x = \frac{Q_x}{\zeta} - \frac{Q}{\zeta^2} \zeta_x, \quad U_y = \frac{Q_y}{\zeta} - \frac{Q}{\zeta^2} \zeta_y$$
 (28)

$$V = \frac{R}{\zeta}, \quad V_t = \frac{R_t}{\zeta} + \frac{R}{\zeta^2} (Q_x + R_y), V_x = \frac{R_x}{\zeta} - \frac{R}{\zeta^2} \zeta_x, \quad V_y = \frac{R_y}{\zeta} - \frac{R}{\zeta^2} \zeta_y$$
 (29)

where in the second equations of (28) and (29), the transformed form of the continuity equation (27) is used. In terms of Q and R in place of U and V, respectively, equations (23) and (24) become

$$\zeta^{2}Q_{t} + Q(2\zeta Q_{x} - Q\zeta_{x} + \zeta R_{y})
+ R(\zeta Q_{y} - Q\zeta_{y}) + \frac{2}{5}\zeta^{2}(Q^{2}\zeta_{xxx} + R^{2}\zeta_{xyy}) + \frac{4}{5}\zeta^{2}(QQ_{x}\zeta_{xx} + RR_{x}\zeta_{yy})
+ \frac{7}{22}\zeta^{4}(\zeta(\zeta_{xxx} + \zeta_{xyy}) + 3\zeta_{x}(\zeta_{xx} + \zeta_{yy})) + \zeta^{3}\zeta_{x} + \frac{m^{2}Q\sqrt{Q^{2} + R^{2}}}{\zeta^{1/3}} = 0$$
(30)

and

$$\zeta^{2}R_{t} + \zeta(RQ_{x} + QR_{x} + 2RR_{y})
-R(Q\zeta_{x} + R\zeta_{y}) + \frac{2}{5}\zeta^{2}(Q^{2}\zeta_{xxy} + R^{2}\zeta_{yyy}) + \frac{4}{5}\zeta^{2}(QQ_{y}\zeta_{xx} + RR_{y}\zeta_{yy})
+ \frac{7}{22}\zeta^{4}(\zeta(\zeta_{xxy} + \zeta_{yyy}) + 3\zeta_{y}(\zeta_{xx} + \zeta_{yy})) + \zeta^{3}\zeta_{y} + \frac{m^{2}R\sqrt{Q^{2} + R^{2}}}{\zeta^{1/3}} = 0$$
(31)

For numerically solving the partial differential equations (29),(30 (31) an invertibility assumption is now made as in Bose [14]. Since the solution of these equations is of the form $\zeta = \zeta(x, y, t)$, $Q = \zeta(x, y, t)$

Q(x, y, t), R = R(x, y, t), if the inverse of these functions exist then, $x = x(\zeta, Q, R)$, $y = y(\zeta, Q, R)$, $t = t(\zeta, Q, R)$. A sufficient condition for the inversion to be possible is that the Jacobian of the transformation does no vanish at points within the domain. Under this assumption, $Q = Q[x(\zeta, Q, R), y(\zeta, Q, R), t(\zeta, Q, R)]$ (a function of ζ , Q, R) and similarly, $R = R[x(\zeta, Q, R), y(\zeta, Q, R), t(\zeta, Q, R)]$ (also a function of ζ , Q, R). The fixed point solution of this iterative pair of equations, for each ζ , by elimination of R and Q, respectively, is of the form

$$Q = F(\zeta), \quad R = G(\zeta) \tag{32}$$

provided the functions F and G are Lipschitz continuous. Physically as was observed in Bose [14], Eq. (32) implies that the discharge components Q and R at a point (x, y) and at time t depends solely on the local elevation ζ at that point and time, as in the theory of gradually varying long tidal waves. The numerical method developed here may therefore be considered as an extension of that theory to the case of two-dimensional turbulent waves. Substituting the forms (32) in equations (27) yields the linear conservation equation

$$\zeta_t + F_x(\zeta) + G_y(\zeta) = 0 \tag{33}$$

The same substitution in Eqs. (30), (31), however, yields a pair of coupled second degree equations in the derivatives $F'(\zeta)$ and $G'(\zeta)$, where the primes denote as usual differentiation with respect to the argument. It is difficult to solve the pair of equations algebraically.

In the important case of almost linear crests propagating in the direction of the *x*-axis, however, $|\zeta_y| << 1$ and |G| << |F| so that the two equations can be solved approximately. To the first order of ζ_y and G, one gets

$$a F'^2 - b F F' + c = 0 (34)$$

and

$$G' = \frac{1}{\zeta_{x} (\zeta F' - F)} \left[\zeta_{x} F' G - \frac{\zeta_{x} F G}{\zeta} + \frac{2}{5} \zeta \zeta_{xxy} + \frac{4}{5} \zeta \zeta_{y} \zeta_{xx} F F' + \frac{7}{22} \zeta^{3} \{ \zeta (\zeta_{xxy} + \zeta_{yyy}) + 3 \zeta_{xx} \zeta_{y} \} \right]$$

$$+ \zeta^{2} \zeta_{y} + \frac{m^{2} F G \operatorname{sgn}(F)}{\zeta^{4/3}}$$
(35)

where

$$a = \zeta^{2}, b = 2\zeta \left(1 + \frac{2}{5} \zeta \zeta_{xx} \right), \quad c = \left(1 - \frac{2}{5} \zeta^{2} \frac{\zeta_{xxx}}{\zeta_{x}} \right)$$
$$- \frac{m^{2} \operatorname{sgn}(F)}{\zeta_{x} \zeta^{1/3}} F^{2} - \frac{7}{22} \zeta^{4} \left[\frac{\zeta}{\zeta_{x}} (\zeta_{xxx} + \zeta_{xyy}) + 3(\zeta_{xx} + \zeta_{yy}) \right] - \zeta^{3}$$
(36)

Equations (33) (34) and (35) are now treated numerically, explained stepwise in the next section.

7 The Numerical Procedure

The initial form of the surface elevation ζ at time t=0 is assumed given. The initial value of the functions F and G are assumed to be estimated from the following consideration. Since the crests are nearly parallel to the y-axis, G is negligible. If moreover, the crests of the waves are assumed to move with constant sub-critical velocity to avoid *choking of flow*, then Eq. (33), $F'(\zeta) = c_0(<1)$ so that $F(\zeta) = c_0\zeta$. Now, as the discharge Q or $F(\zeta)$ is an increasing function of ζ , the appropriate solution of the quadratic equation (34) is

$$F' = \frac{bF + \sqrt{b^2 F^2 - 4ac}}{2a} \tag{37}$$

For real solution $b^2F^2 \ge 4ac$ representing undulating flow surface; otherwise F'=0 for steady discharge. Eq. (37) together with Eq. (35) for evolving value of ζ is next inegrated by the trapezoidal rule based iterative, second-order, modified Euler method (Bose [15], p. 264). The coefficients a, b, c given in Eq. (36) contain spatial partial derivatives of ζ up to the third order. These derivatives are calculated by second-order finite differences of ζ over a grid of points $x = (m-1) \Delta x$, $y = (n-1) \Delta y$, $(m=1, 2, 3, \dots; n=1, 2, 3, \dots)$,

where Δx , Δy are the grid lengths parallel to the x and y - axes, respectively. These derivatives are thus computed by the finite difference formulae:

$$\zeta_x = \frac{\zeta(x + \Delta x, y) - \zeta(x - \Delta x, y)}{(\Delta x)^2}$$
(38)

$$\zeta_{xx} = \frac{\zeta(x + \Delta x, y) - 2\zeta(x, y) + \zeta(x - \Delta x, y)}{(\Delta x)^2}$$
(39)

$$\zeta_{xxx}(x, y) = \frac{1}{\Delta x^3} [\zeta(x + 2\Delta x, y) - 3\zeta(x + \Delta x, y) + 3\zeta(x, y) - \zeta(x - \Delta x, y)]$$

$$\zeta_{xxy}(x, y) = \frac{1}{2(\Delta x)^3} [\zeta(x + \Delta x, y + \Delta y) - \zeta(x + \Delta x, y - \Delta y) - 2\zeta(x, y + \Delta y) + 2\zeta(x, y - \Delta y)$$
(40)

$$+\zeta(x - \Delta x, y + \Delta y) - \zeta(x - \Delta x, y - \Delta y)] \tag{41}$$

and similar expressions for $\zeta_y(x, y), \zeta_{yy}(x, y), \zeta_{yyy}(x, y)$, and $\zeta_{xyy}(x, y)$, for x, y > 0 or, $m, n \ge 2$. For x = 0 and y = 0, linear extrapolations

$$\zeta_x(0, y) = 2\zeta_x(\Delta x, y) - \zeta_x(2\Delta x, y) \tag{42}$$

$$\zeta_{\mathbf{x}}(\mathbf{x}, 0) = 2\,\zeta_{\mathbf{x}}(\mathbf{x}, \,\Delta\mathbf{y}) - \zeta_{\mathbf{x}}(\mathbf{x}, \,2\Delta\mathbf{y}) \tag{43}$$

and similar expressions for the other partial derivatives are used. The value of F and G at the grid points are thus obtained by the recursive formulae:

$$F(m+1, n) = F(m, n) + \Delta x \times [F'(m, n) \zeta_x(m, n)] \leftarrow \frac{1}{2} [F(m, n) + F(m+1, n)]$$
(44)

$$G(m, n+1) = G(m, n) + \Delta y \times [G'(m, n) \zeta_y(m, n)] \leftarrow \frac{1}{2} [G(m, n) + G(m, n+1)]$$
 (45)

Equations (44) and (45) determine the values of the functions F and G for a computed elevation ζ at subsequent time t. The evolution of ζ as solution of Eq. (33) is obtained by application of the second-order Lax–Richtmyer scheme for increasing time $t = l \Delta t$, ($l = 1, 2, 3, \cdots$). According to Richtmyer and Morton [16], the time evolution of ζ from given step l is obtained in two steps l + 1 and l + 2:

$$\zeta_{m,n}^{l+1} = \frac{1}{4} \left(\zeta_{m+1,n}^l + \zeta_{m-1,n}^l + \zeta_{m,n+1}^l + \zeta_{m,n-1}^l \right) - \frac{\Delta t}{2 \Delta x} \left(F_{m+1,n}^l - F_{m-1,n}^l \right) - \frac{\Delta t}{2 \Delta y} \left(G_{m,n+1}^l - G_{m,n-1}^l \right)$$
(46)

where $F_{m,n}^l = F(\zeta_{m,n}^l)$, and at the second step

$$\zeta_{m,n}^{l+2} = \zeta_{m,n}^{l} - \frac{\Delta t}{\Delta x} \left(F_{m+1,n}^{l+1} - F_{m-1,n}^{l+1} \right) - \frac{\Delta t}{\Delta y} \left(G_{m,n+1}^{l+1} - G_{m,n-1}^{l+1} \right)$$
(47)

Equation (47) determines the elevation at the next time step. In practice, one can take $\Delta x = \Delta y$ and $\Delta t = r \Delta x$, where the factor r < 1 is so chosen that the computation remains stable and convergent for small changes in its value.

Following the method developed above, the algorithm of computation of the elevation ζ becomes:

Algorithm: Computation of free surface elevation $\zeta(x, y, t)$.

Step 1. Set maximum values mmax of m, nmax of n, and lmax of l. Set the values of Δx , Δy , r and $\Delta t = r \Delta x$. Set the value of c_0 .

Step 2. For m = 1, mmax - 2, n = 1, nmax - 2Compute the initial elevation $\zeta(m, n)$ at the grid point (m, n) from a given initial free surface elevation profile at time t = 0.

Step 3. Set $l \leftarrow 1$

Step 4. For m=1, mmax-2, n=1, nmax-2Compute the spatial derivatives ζ_x etc. using Eqs. (38)–(41). Similarly for ζ_y etc. End For

Step 5. For
$$m = 1$$
, $mmax - 2$, $n = nmax - 2$
Compute $\zeta_x(x, 0)$, $\zeta_x(0, y)$ etc. using Eqs. of the type (42) and (43)..
End For

Step 6. For
$$n = 1$$
, $nmax - 2$, $m = 1$, $mmax - 2$
Compute a , b , c using Eq. (36) and $F(m + 1, n)$ by say 10 iterations using Eq. (44). Similarly compute $G(m, n + 1)$ by using Eqs. (45) and (35). End For

Step 7. For
$$m = 2$$
, $mmax - 2$, $n = 2$, $nmax - 2$
Compute $\zeta(m, n)$ using Eqs. (46) and (47).
Print $\zeta(m, n)$ at suitable grid points (m, n) or (x, y) .
If $(l > lmax)$ Stop
End For

Step 8.
$$l \leftarrow l+1$$
; $t \leftarrow l \times \Delta t$
Go To Step 4

8 Application to a Model Initial Surface Elevation

The numerical method of the preceding section is applied to a model initial surface elevation considered by Howe [17], p.332 for tsunami propagation caused by sudden uplifting of the sea bed in the form of a ridge caused by subduction of the bed in front. Accordingly the initial elevation of the sea surface is assumed to follow the dislocated bed form with a profile given by the equation

$$\zeta = 1 + \zeta_0 \left[\frac{1}{\{1 + [(x+L)^2 + y^2]/L^2\}^{3/2}} - \frac{1}{\{1 + [(x-L)^2 + y^2]/L^2\}^{3/2}} \right]$$
(48)

where ζ_0 = maximum elevation/depression above/below the mean surface of water, and 2L = distance between the highest and the lowest points of the initial free surface. For presentable results, it is assumed that $\zeta_0 = 0.3$ and L = 10. The profile tapers to plane level $\zeta = 1$ as |y| tends to infinity. The initial value of ζ at t = 0 is computed for x, $y \ge 0$ from Eq. (48), taking $\Delta x = \Delta y = 0.1$ and $\Delta t = r \Delta x$, where r = 1/2. The

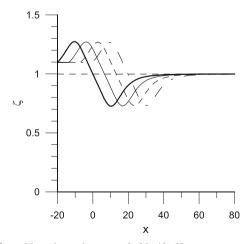


Fig. 1 View of propagating surface from 0^o angle, at times t = 0, 20, 40, 60

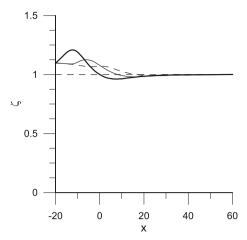


Fig. 2 View of propagating surface from 26.57° angle, at times t = 0, 20, 40, 60

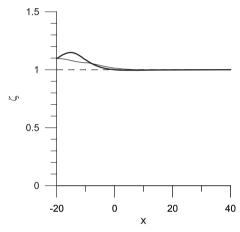


Fig. 3 View of propagating surface from 45° angle at times t = 0, 20, 40

numerical scheme is found to yield stable and convergent values of ζ for small changes in the value of r. The time evolution of the elevation ζ following the Lax–Richtmyer scheme (46), (47) is then carried out using the algorithm developed in the preciding section. Performing a large number of iterations over time, the computed results for ζ at times t=0, 20, 40 and 60 with respect to x for the three representative cases y=0, $y=\frac{1}{2}x$, and y=x are presented in Figs. 1, 2, and 3, respectively. The three cases correspond to the crests being viewed from the angles 0^o , 26.57^o and 45^o , respectively, to the crests. It is found in the computed data that the crest height diminishes slowly with time with corresponding rise in the trough depth. This feature diminishes with the increase in the angle, becoming insignificant in the third case.

The method can be employed to generate data for any other cross section.

A qualitative comparison of the results presented above can be made with those of Howe [17], presented on page 333 of the text. Based on potential surface wave theory, the surface elevation ζ is given by the Eq. (5.7.7), p. 330 of that text. The notable difference between the two approaches is that while in the theory presented here, the wave form realistically leans forward as it propagates that feature is not replicated in the surface wave theory presented in the text of Howe [17].

9 Conclusion

In this paper, the turbulent shallow water free surface flow equations in two dimensions are developed and numerically integrated by a method developed in Bose [14]. Following Bose and Dey [12], the basic equations of continuity and momentum conservation for the flow are developed from the Reynolds Averaged Navier–Stokes equations. The equations are expressed in terms of the depth averaged horizontal velocity (U, V) and

the surface elevation ζ , all being functions of x, y and time t. Based on certain assumptions necessitated by the presence of turbulence, three nonlinear differential equations are obtained for the determination of ζ , U and V. For numerical treatment of the equations, a slight simplification is made in the two horizontal momentum equations where in the terms arising from instantaneous vertical acceleration, the quantities $\partial U/\partial t$, $\partial V/\partial t$ and $\partial U/\partial x + \partial V/\partial y$ are replaced by equivalent expressions in terms of ζ obtained by linearizing the two equations. A transformation of the equations is then made by replacing U and V by discharge components Q and R, respectively. An invertibility argument of the functions shows that Q and R can be functions of ζ , say $Q = F(\zeta)$ and $R = G(\zeta)$, as in the theory of long tidal waves. The method may therefore be considered an extension of the linear inviscid long wave theory to nonlinear waves taking into account the generated turbulence in real fluids. Substitution in the continuity equation leads to a conservation equation of standard form, while the two momentum equations yield coupled second degree equations in the derivatives $F'(\zeta)$ and $G'(\zeta)$. In the important case of propagation of nearly linear crested waves on the water surface parallel to y-axis, $F'(\zeta)$ is given to a first order by a decoupled quadratic equation, and $G'(\zeta)$ by an expression containing $F(\zeta)$ and $F'(\zeta)$. The two equations are numerically treated to compute $F(\zeta)$ and $G(\zeta)$, following which the standard form mass conservation equation is numerically solved by the Lax-Richtmyer scheme. The method is implemented for an almost linear initial bed elevation model given in Howe [17] for simulating tsunami propagation. The computed values are shown for three different angles of view, showing very slowly decreasing wave height. The data generated by method opens up the possibility of creating animation of the moving free surface for greater visual effect. The numerical method possibly can also be employed to study atmospheric flows and oscillations.

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Declarations

Conflict of interest There is no conflict of interest involved in the reported research of this paper.

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